

# Flexural Deformation of Expanded Polystyrene Concrete Sandwich Beam: An Experimental Study

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## Abstract

Expanded Polystyrene (EPS) concrete sandwich structure has been developed and investigated since few years ago. It has advantage of high heat resistance, impact, acoustic and vibration absorption, and easy to built. However, it is important to make benchmarking of the local product of (EPS) concrete sandwich structure and the global product. Therefore, this research aims to investigate the flexural deformation of local product of EPS concrete sandwich beams such as its deflection, peak load, and flexural strength. There are 9 EPS concrete sandwich beams were tested for flexural strength with dimension of L x 200 x 7.5 mm, where L were three specific lengths of 50, 100, and 100 mm. The flexural test carried out by Universal Testing Machine, LVDT, and data logger. The experiment has shown that Optimum values of maximum deflection, peak load, and flexural strength, achieved by series II of the specimens. There was a “jigsaw phenomenon” during the loading of series II and III. Optimum values of EPS concrete sandwich beam achieved by series II with length of 100 mm. It has peak load of 4.39 kN, flexural strength of 3.42 MPa and maximum deflection of 2.47 mm. The research found that the local product of EPS concrete sandwich beam used as specimens in this research complies with the previous investigation and application.

**Keywords:** flexural, deformation, expanded polystyrene, concrete, sandwich beam.

## I. INTRODUCTION

Concrete sandwich structure has been developed since many decades ago due to its high heat resistance, impact, acoustic and vibration absorption, and easy to built [1]. This type of concrete fulfilled the need of lightweight materials for high rise building and still developed until nowadays. First studies by [2] and [3] delivered theories of sandwich structures which previously were implemented by application of Expanded Polystyrene (EPS) in 1957[4]. It was EPS concrete sandwich panel that used because of its advantage of low density and best insulation ability. Instead of the EPS concrete sandwich can transfer load and insulate structure perfectly, it has low compressive strength as about less than 10 MPa. Several

researches have been carried out to develop EPS and other materials for concrete sandwich structures such as [5–13].

Previous investigations have been developed and studied the performance of EPS concrete sandwich structure, but little information available about the study of local manufactures EPS concrete sandwich structure. It is important to make benchmarking of the quality of the EPS concrete sandwich structure to the global product, hence this research conducted especially in investigating the flexural deformation of local product of EPS concrete sandwich beams. This research aims to study deflection, peak load, and flexural strength of EPS concrete sandwich beams that expected meets the quality of the global product.

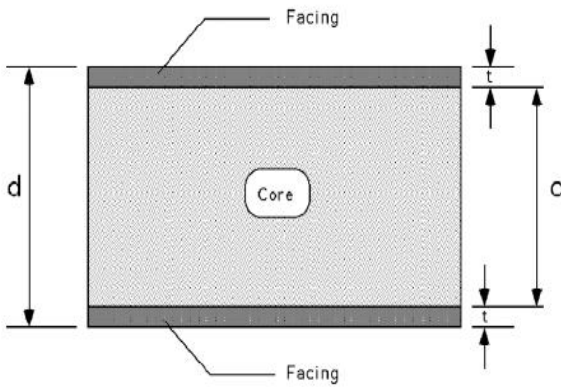
## II. MATERIALS AND METHODS

### II.I. Materials

EPS concrete sandwich used in this research was a panel of 240 x 61 x 7.5 m<sup>3</sup> which was cut and used as beam. The appearance of a EPS concrete sandwich block shown by Fig that consist of facings (skins) and core as described by Fig 2 [14]. The height of EPS concrete sandwich beam was 7.5 m with facing width of 6 mm.



Fig 1. EPS concrete sandwich block



**Fig 2.** EPS concrete sandwich block cross-section

There were 9 beam specimens prepared for flexural test of third point loading that referred to ASTM C393/C393M Standard Test Method for Core Shear Properties of Sandwich Constructions by Beam Flexure. Dimension for beam specimens described by Table 1.

**Table 1.** EPS concrete sandwich beam dimension

specimen series	length (L)	width (b)	height (h)	total specimen
	(mm)	(mm)	(mm)	(pcs)
I	500	200	75	3
II	1,000	200	75	3
III	1,500	200	75	3

### II.II. Experimental Program

Flexural Test for beam specimens was carried out by loading frame and also Universal Testing Machine (UTM) of Hung Ta HT-EH 21016 with application of Linear Variable Displacement Transducer (LVDT) and Data Logger of Schumy Data Logger TML, Type S-2251 (Fig 3). UTM was recording displacement, load, and time during the test.

Flexural strength of beams was calculated by Equation (1) that referred to Indonesia National Standard of SNI-03-4431-1997 Methods of Normal Flexural Test of Two Points Loading as follow.

$$\sigma = \frac{P.L}{b.h^2} \quad (1)$$

Where:

- $\sigma$  = flexural strength of beam (MPa)
- P = load (kN)
- L = beam span (mm)
- b = width of horizontal cross-section (mm)
- h = height of vertical cross-section (mm)
- a = average value of space between cracked cross-section and nearest outer support (mm)



**Fig 3.** Universal Testing Machine with LVDT and Data Logger

### III. RESULT

All specimens were successfully tested for flexure strength as shown by Fig 4. Flexural test was carried out until beam specimens were getting collapse (Fig 5) and broken into pieces (Fig 6). Load-time, load-deflection, and deflection-time relations of the beam specimens will be explained as follow.



**Fig 4.** Flexural Test of Specimens IIIA



**Fig 5.** Beam specimen was getting collapse after reach maximum deflection

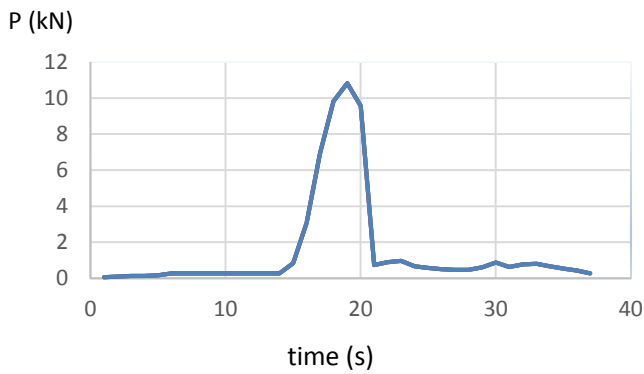


**Fig 6.** Crack pattern of collapsed beam specimen

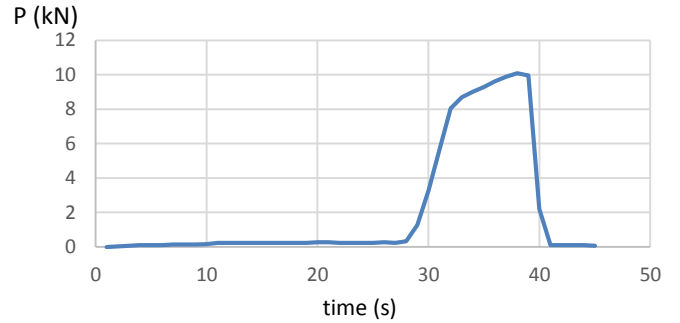
Specimen IA achieved peak load of 0.262 kN at 18 seconds and had deflection of 6.658 mm at 37 seconds as described by Fig 7., Fig 8., and Fig 9. First crack appeared at 17 seconds that shown by Fig 9. There was about 20 seconds prior to the time of maximum deflection (Fig 7) achieved after the peak load (Fig 8).

**III.II. Specimen IB**

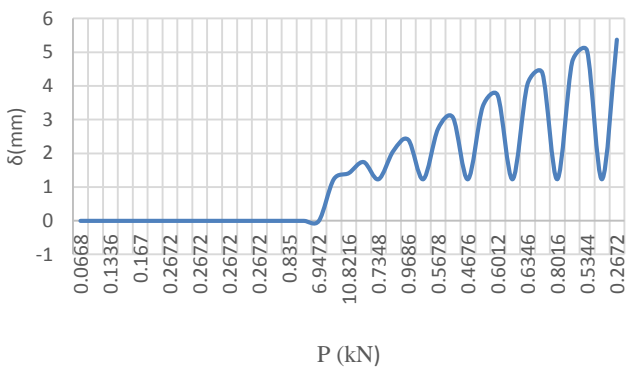
**III.I. Specimen IA**



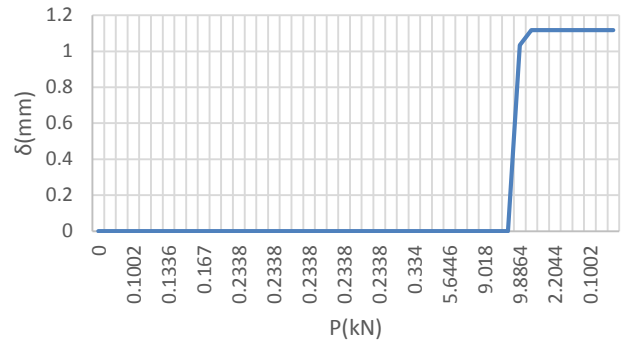
**Fig 7.** Load-time relation of specimen IA



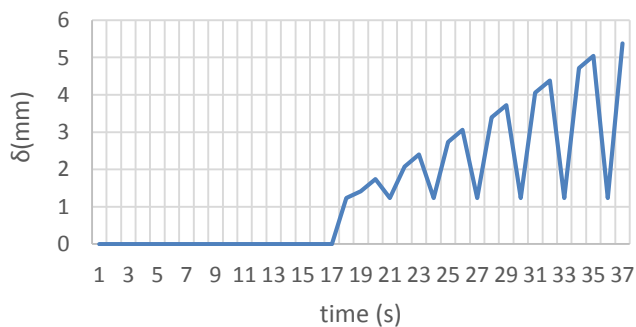
**Fig 10.** Load-deflection relation of specimen IB



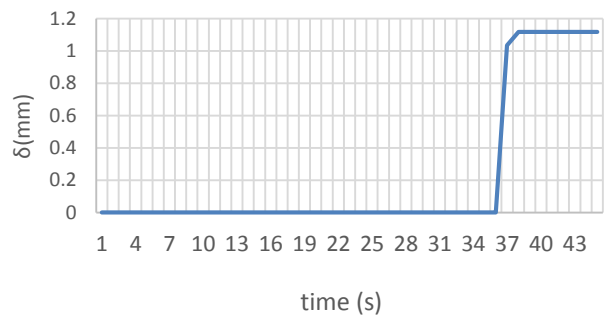
**Fig 8.** Deflection-stress relation of specimen IA



**Fig 11.** Load-time relation of specimen IB



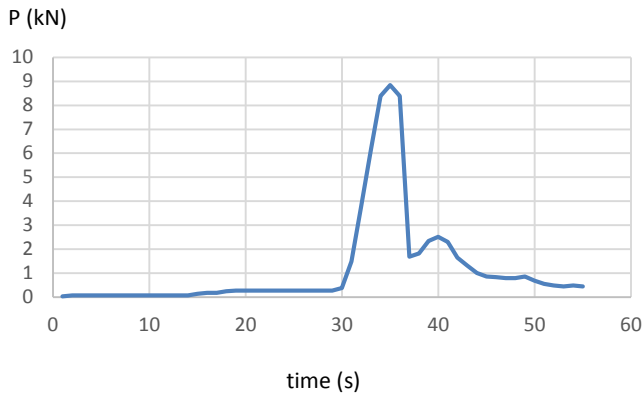
**Fig 9.** Deflection-time relation of specimen IA



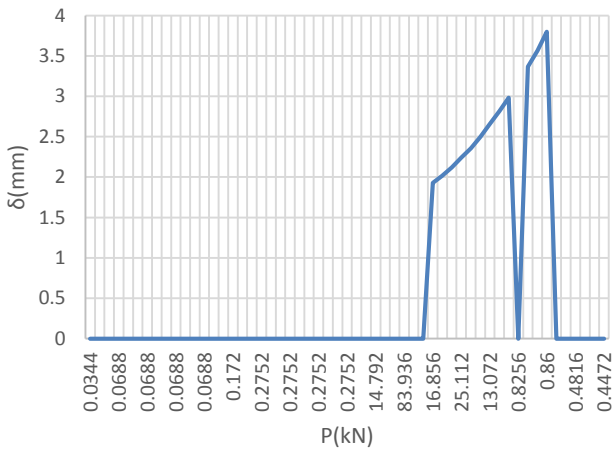
**Fig 12.** Deflection-time relation of specimen IB

Specimen IB performed a little bit different behaviour compared to specimen IA. Peak load of 10.0868 kN was achieved at 38 seconds while maximum deflection of 3.26 mm existed at 37 seconds as described by Fig 10., Fig 11., and Fig 12. First crack appeared at 27 seconds (Fig 10) and it was just jumped up to peak load at 37 seconds that shown by Fig 11 and Fig 12. There was only about 7 seconds before specimen was getting collapsed.

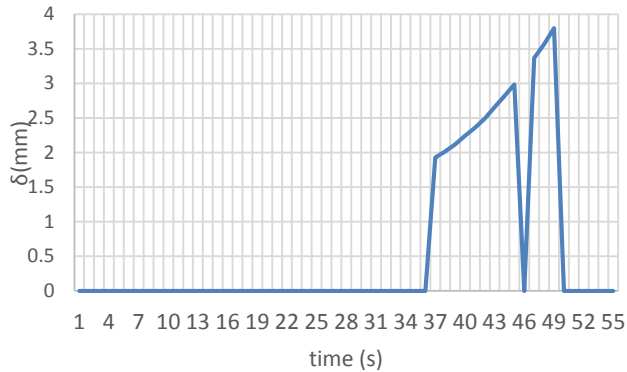
**III.III. Specimen IC**



**Fig 13.** Load-deflection relation of specimen IC



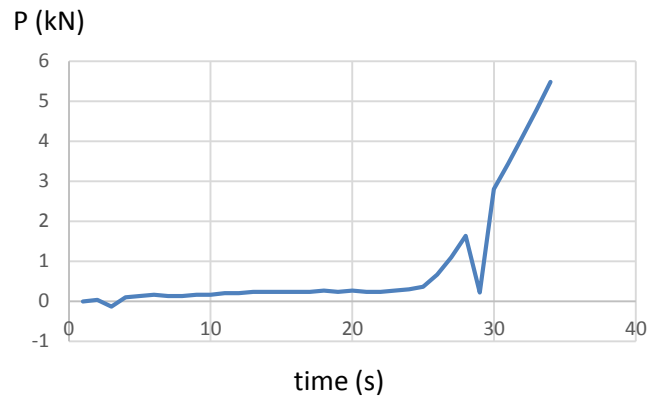
**Fig 14.** Load-time relation of specimen IC



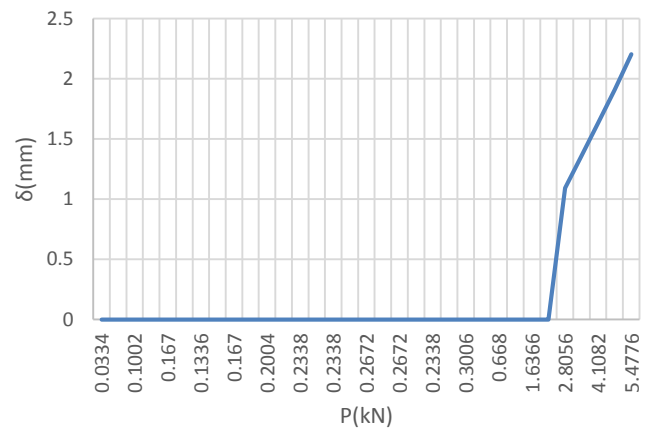
**Fig 15.** Deflection-time relation of specimen IC

The case of specimen IC was similar to specimen IB unless there was longer time before specimen collapsed. Specimen IC achieved peak load of 8.8404 kN at 35 seconds while maximum deflection of 4.03 mm existed at 55 seconds as described by Fig 13., Fig 14., and Fig 15. However, Fig 15 has shown that there was a lag in the end of loading that the load jumped nearly to zero but increased again to achieve peak load before collapsed.

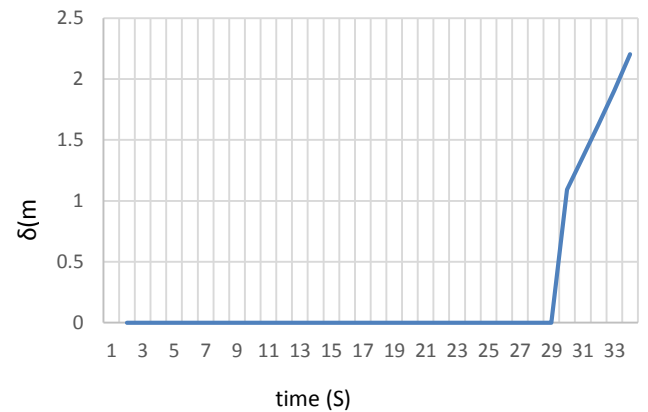
**III.IV. Specimen IIA**



**Fig 16.** Load-deflection relation of specimen IIA



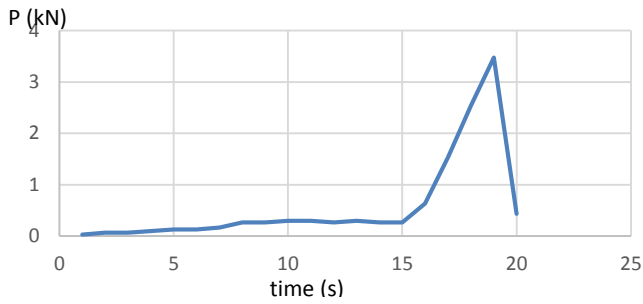
**Fig 17.** Load-time relation of specimen IIA



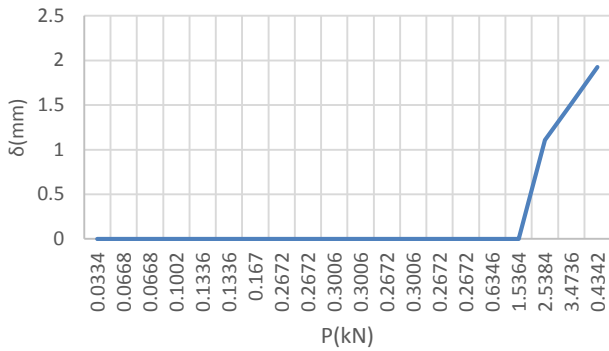
**Fig 18.** Deflection-time relation of specimen IIA

The series of II specimens were twice longer than the series of I specimens. Hence, different performance has been established. Specimen IIA achieved peak load of 5.4776 kN at 34 seconds while maximum deflection of 2.204 mm existed as described by Fig 16., Fig 17., and Fig 18. It was shown by Fig 16 that cracks existed at 4 seconds and right after the peak load achieved and the specimen collapsed.

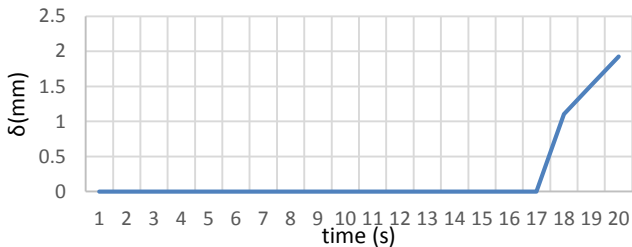
### III.V. Specimen IIB



**Fig 19.** Load-deflection relation of specimen IIB



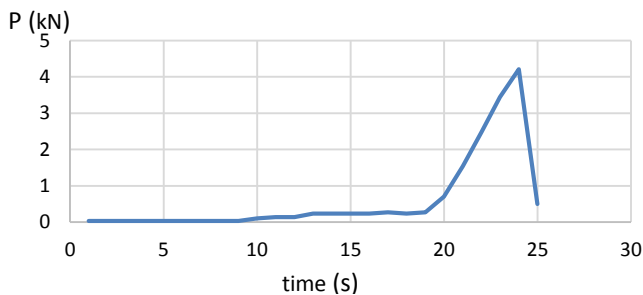
**Fig 20.** Load-time relation of specimen IIB



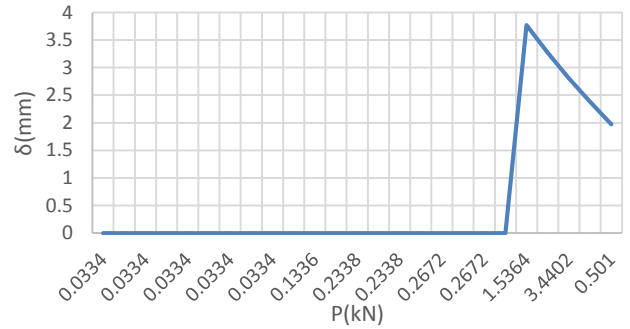
**Fig 21.** Deflection-time relation of specimen IIB

Specimen IIB had shorter time (19 seconds) to collapse compared to specimen IIA. It achieved peak load of 3.4736 kN at 19 seconds while maximum deflection of 1.926 mm happened at load of 0.4342 kN as described by Fig 19., Fig 20., and Fig 21. It was shown by Fig 16 that right after the peak load achieved the specimen collapsed.

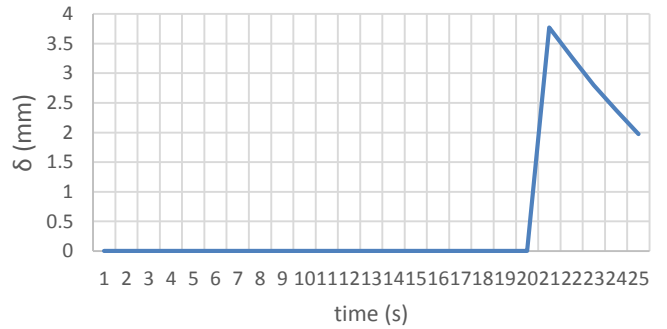
### III.VI. Specimen IIC



**Fig 22.** Load-deflection relation of specimen IIC



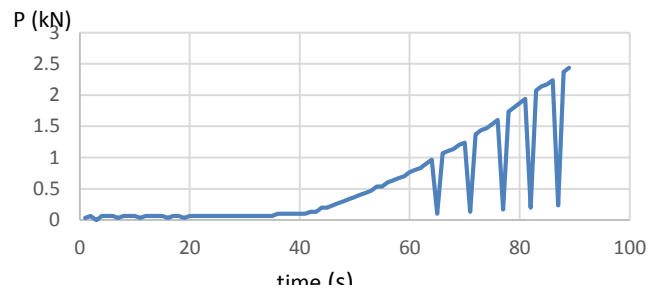
**Fig 23.** Load-time relation of specimen IIC



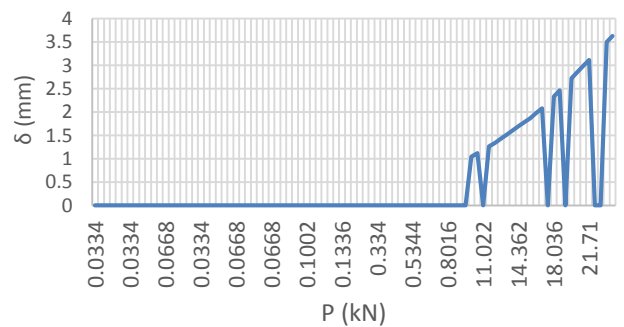
**Fig 24.** Deflection-time relation of specimen IIC

Even though specimen IIB has short period of collapse, specimen IIC performed better. The peak load of 4.2 kN achieved by specimen IIC at 24 seconds while maximum deflection of 4.236 mm existed at load of 1.5 kN (Fig 22., Fig 23., and Fig 24.). Time to specimen collapsed was 35 seconds.

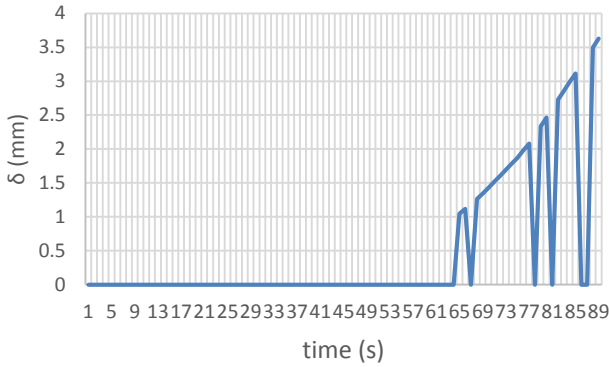
### III.VII. Specimen IIIA



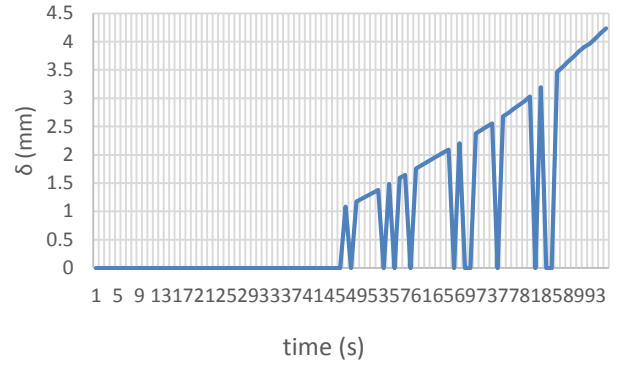
**Fig 25.** Load-time relation of specimen IIIA



**Fig 26.** Load-deflection relation of specimen IIIA



**Fig 27.** Deflection-time relation of specimen IIIA

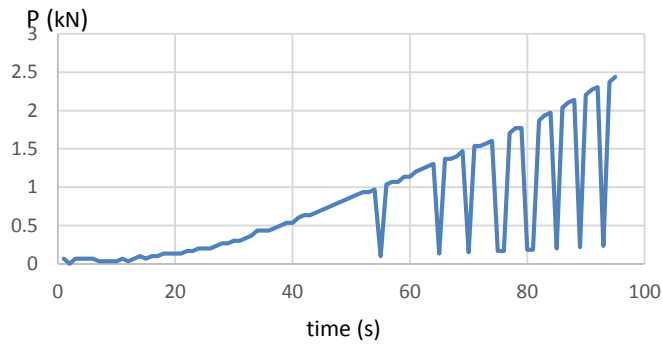


**Fig 30.** Deflection-time relation of specimen IIIB

The series of III of specimens were three times longer than the series of I specimens to get collapsed. Therefore, there was significant behaviour existed during the loading. Specimen IIIA performed gradual increase of load at 40 seconds to 88 seconds (Fig 25) with “jigsaw phenomenon” (Fig 26 and Fig 27). Specimen IIIA achieved peak load of 2.3724 kN at 88 seconds while maximum deflection of 3.638 mm.

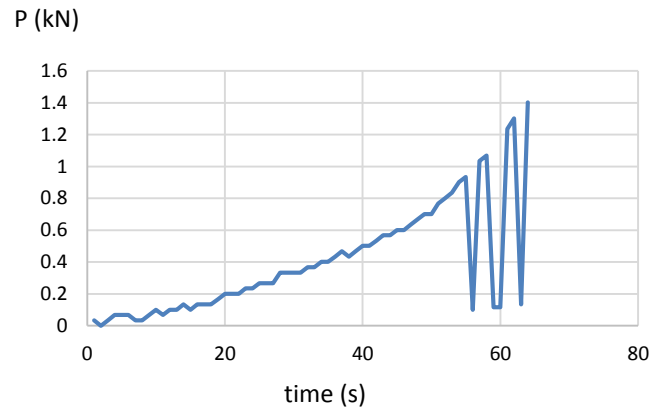
It was found that specimen IIIB had longer time (95 seconds) to collapse compared to specimen IIIA. It achieved peak load of 2.382 kN at 95 seconds while maximum deflection of 4.0512 mm as described by Fig 28., Fig 29., and Fig 30. Gradual load increase was established from the initial of loading (Fig 28.).

**III.VIII. Specimen IIIB**

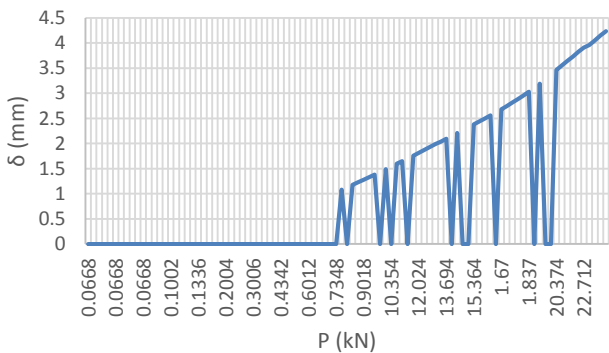


**Fig 28.** Load-deflection relation of specimen IIIB

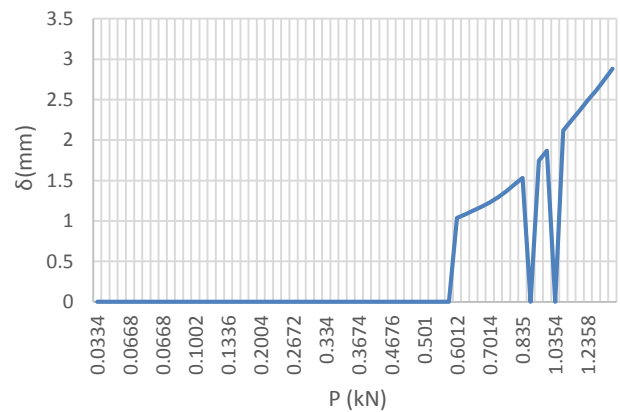
**III.IX. Specimen IIIC**



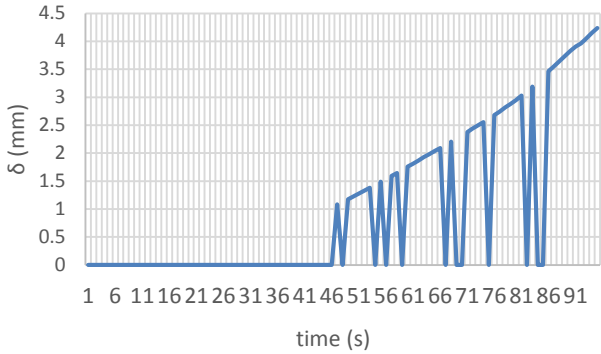
**Fig 31.** Load-deflection relation of specimen IIIC



**Fig 29.** Load-deflection relation of specimen IIIB



**Fig 32.** Load-deflection relation of specimen IIIC



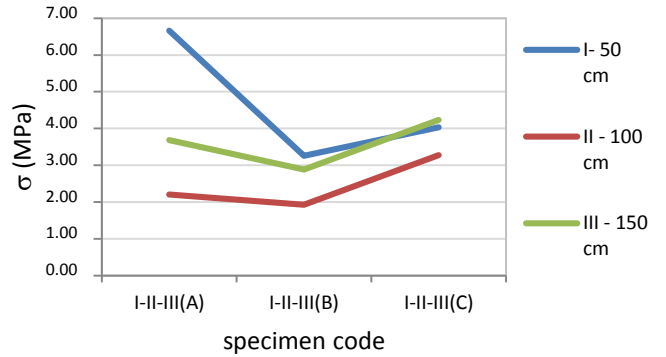
**Fig 33.** Deflection-time relation of specimen IIIC

Specimen IIIC performed shorter time to collapse (64 seconds) as shown by Fig 31. Similar to specimen IIA, right after achieving peak load 1.4028 kN, specimen IIIC was collapsed (Fig 31.) Maximum deflection of 4.056 mm existed at 95 seconds. Phenomenon of “jigsaw” also existed during the loading (Fig 32 and Fig 33).

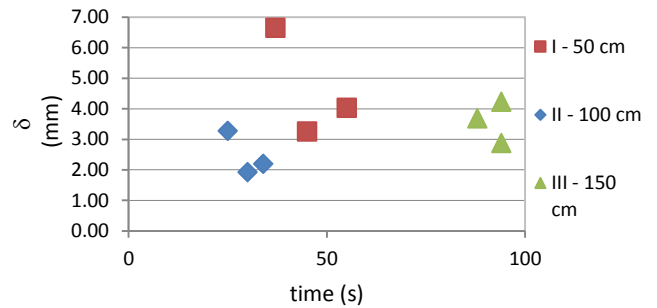
**V. DISCUSSION**

Flexural test has been carried out for all specimens and the flexural strength were calculated based on the experimental result as detailed by Table 2. Highest flexural strength was achieved by series II (3.42), follow by series I (3.06), and series III (2.67). Results of Fig 7 to Fig 33 have shown that the longer specimens the lower peak load can be achieved. However, deflection of the specimen series performed non-linear behaviour since the lowest deflection achieved by series II, followed by series III and series I as described by Fig 34. It is also found that deflection-time relation specimen series was not linear relation. Series II has shorter time of maximum deflection, followed by series I and series III.

Optimum values of maximum deflection, peak load, and flexural strength, achieved by series II of the specimens. This finding confirms the study [15] of that concrete sandwich beam (2.5 m x 20 cm x 20 mm) containing EPS (and also Textile Reinforced Cementitious Composites, TRCs) can have maximum deflection about 45 mm and peak load about 5 kN. Hence, the optimum length of EPS concrete sandwich beam is 100 mm, which can be implemented to field application.



**Fig 34.** Deflection of the specimen series



**Fig 35.** Deflection-time relation of the specimen series

**Table 2.** Flexural strength of EPS sandwich beam specimens\*

specimen code	load (P)	length (L)	width (b)	height (h)	weight	span between two supports	deflection (delta)	flexural strength (sigma)	flexural strength (sigma) average
	(N)	(mm)	(mm)	(mm)	(kg)	(mm)	(mm)	(MPa)	(MPa)
IA	10.82	500	200	75	5.40	350	6.66	3.38	3.06
IB	10.09	500	200	75	5.51	350	3.26	3.15	
IC	8.84	500	200	75	4.89	350	4.03	2.63	
IIA	5.48	1,000	200	75	9.62	850	2.20	4.24	3.42
IIB	3.47	1,000	200	75	10.64	850	1.93	2.73	
IIC	4.21	1,000	200	75	10.26	850	3.27	3.28	
IIIA	2.44	1,500	200	75	15.85	1350	3.68	3.09	2.67
IIIB	2.44	1,500	200	75	15.68	1350	2.88	3.09	
IIIC	1.03	1,500	200	75	15.47	1350	4.24	1.85	

\*Reported by authors in [5]

## VI. CONCLUSION

This research found that flexural deformation of EPS concrete sandwich beam will perform optimum by the length of 100 mm. It has peak load of 4.39 kN, flexural strength of 3.42 MPa and maximum deflection of 2.47 mm. However, the authors suggest thicker facing of EPS concrete sandwich beam and better quality of epoxy to increase flexural strength and bonding mechanism. After all, the local product of EPS concrete sandwich beam used as specimens in this research complies with the previous investigation and application.

## ACKNOWLEDGMENT

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